MIDLANTIC ENGINEERING, INC. 120 COMMERCE ROAD PITTSTON TOWNSHIP, PA 18640-9552 GEOTECHNICAL ENGINEERING REPORT STORMWATER MANAGEMENT AND INFILTRATION TESTING MILFORD WAREHOUSE DEVELOPMENT MILFORD TOWNSHIP, PA (Project #23004-SWM) PREPARED FOR:

GLEN MILLS, PA 19342

NATIONAL LAND DEVELOPERS, LLC 1010 WILSON AVENUE

AUGUST 18, 2023



August 18, 2023

National Land Developers, LLC 1010 Wilson Avenue Glen Mills, PA 19342

ATTENTION: Mr. Joseph Marley

jmarley@nationalld.com

REFERENCE: GEOTECHNICAL ENGINEERING REPORT (#23004-SWM)

Stormwater Management and Infiltration Testing

Milford Warehouse Development

Milford Township, PA

Mr. Marley:

Submitted herewith is our geotechnical engineering report to address the proposed stormwater management areas for the above referenced development. Our services have been performed in accordance with our proposal/agreement dated October 24, 2022 and your subsequent authorization to proceed.

1. SCOPE OF SERVICES

Services performed for this phase of the study included site reconnaissance, observation and logging of seventeen test pits, in-situ infiltration testing, soil laboratory analysis, and preparation of our report. Our geotechnical engineering analysis and report for the proposed stormwater management development areas includes the following:

- a. Our evaluation of the estimated subsurface conditions within the proposed stormwater management infiltration areas based on the data obtained.
- b. Analysis of subgrade conditions, soil laboratory testing, and in-situ infiltration testing to provide design parameters for infiltration devices.
- c. Comments concerning the use of infiltration practices at the designated areas tested.

Services with respect to environmental considerations, wetlands investigations, erosion control, building foundations, pavement designs, construction cost or quantity estimates, and construction observation and testing are not included in the scope of services under this phase of our contract.

2. DESCRIPTION OF SITE AND PROPOSED CONSTRUCTION

The project area is bordered by I-84 to the north, by SR 6 to the south, and by commercial properties to the west and east. The proposed structure is a 434,000 ft² warehouse structure with a finish floor at El 904. The project vicinity is indicated on a 2023 USGS quadrangle map and on a 2023 aerial photograph included as Figure Nos. 1-1 and 1-2 in Enclosure (1).

Underground and aboveground infiltration basins and a rain garden are proposed to the north, south, and west of the structure. To the east of the structure is a water quality basin (Basin #3). The proposed bottoms of the infiltration basins and rain garden vary.

The information listed above was provided to us by your office, derived from project development plans provided to us, or was obtained during our own site visits.

3. SUBSURFACE CONDITIONS

In order to evaluate subsurface conditions in the proposed stormwater management areas, seventeen test pits were excavated. The test pits were extended to a depth of approximately 4 feet to 18.5 feet to determine the shallow subsurface stratification and depths to limiting zones, if applicable. The test pit logs, water observation data, and test pit location plan are included in Enclosure (4).

3.1 Test Pit Data and Stratification

The test pits indicate the following generalized strata underlie the proposed stormwater management area to the depths investigated:

<u>Stratum</u>	<u>Depths</u>	<u>Description</u>
Stratum F (fill)	from surface grades to depths of 1 to 2 feet at test pits TP-I-1B, TP-I-2B, TP-I-4A, and TP-I-6E	silty sand (SM), poorly graded gravel with sand (GP), and silty sand with gravel (SM) – FILL; includes brick fragments and rock fragments
Stratum B	below topsoil and Stratum F material to maximum depths investigated	silty SAND with gravel (SM), poorly graded SAND with silt (SP-SM), poorly graded SAND with silt and gravel (SP-SM), silty GRAVEL with sand (GM), poorly graded GRAVEL with silt and sand (GP-GM); occasional cobbles

The soil symbols indicated in the stratum descriptions and on the test pit logs represent the Unified Soil Classification (ASTM D-2488) group symbols based on visual observation of the specimens recovered. Criteria for visual classification of soil samples are given in Enclosure (4) of this report. The visual classifications may vary from the results of laboratory testing classifications.

3.2 Geology

The existing fill materials of Stratum F are associated with site grading for previous developments in the area.

The natural materials of Stratum B represent glacial lake delta deposits of the Pleistocene geologic epoch. The natural soil subgrades are moderately over-consolidated. The surficial geology mapping of the study area is included as Figure No. 2-2 in Enclosure (2).

The underlying bedrock consists of interbedded shales and siltstones. The sedimentary rock belongs to the Trimmers Rock formation of the Devonian period. Bedrock was not encountered in the test pits to the maximum depths investigated.

3.3 Groundwater Observations

Groundwater observations were performed during excavation at the test pit locations. The results of the groundwater observations are shown on the test pit logs in Enclosure (4).

A perched water level was encountered at a depth of 3.0 feet in test pit TP-I-6E.

Groundwater was not encountered in the remaining test pits to the maximum depths investigated.

Water level readings at the test pit locations are considered to be a reliable indication of groundwater conditions at the times indicated. Fluctuations in groundwater levels, as well as perched water, may be expected with variations in precipitation, evaporation, adjacent construction activity, and similar factors.

4. INFILTRATION FACILITY ANALYSES

The proposed stormwater management area was identified within the study area and were investigated for suitability for infiltration devices.

The Soil Conservation Service mapping of Pike County was referenced for the development area. The delineation of the development area is overlain on the SCS mapping included as Figure No. 2-1 in Enclosure (2). The mapping indicates the natural soil subgrades of the site generally consist of glacial lake delta material deposits. The engineering index and physical properties of the series provided by SCS are indicated as follows:

Series Designation	USCS Classifications	Hydrologic Classification	Estimated Permeability (in/hr.)
89B – Chenango gravelly fine sandy loam	SM, GM, GP-GM, GW-GM	A	0.6 to 20.0 in/hr.
9B – Craigsville-Wyoming complex	SM, SP-SM, SW-SM, GM GP-GM, GW-GM	A/B	2.0 to 20.0 in/hr.

The protocols outlined in the final draft of the Pennsylvania Stormwater Best Management Practices Manual were used as the reference for design and construction standards for stormwater infiltration systems. The protocols include parameters for the conduct of the infiltration testing, site conditions, design considerations, construction requirements and factors of safety.

4.1 Depth of Limiting Zones

Protocol 2 of the referenced manual recommends that a minimum 2-foot clearance be maintained between the bottom of the infiltration facility and a limiting zone of seasonally high water table or intact bedrock.

Water was encountered at a depth of 3.0 feet in test pit TP-I-6E.

Groundwater was not encountered in the remaining test pits to the maximum depths investigated.

Bedrock was not encountered to the maximum depth investigated.

The proposed infiltration facility can be considered feasible based on depths to limiting zone.

4.2 Soil Textures

Protocol 2 recommends that infiltration facilities be constructed in native soils without prior fill or disturbance. Protocol 2 allows infiltration in areas that have experienced historic fill or disturbance provided that sufficient time has elapsed to restore natural permeability, which is defined as at least 5 years in Chapter 6.

The proposed infiltration facilities were investigated at depths ranging from 0.5 feet to 16.2 feet below surface grade. The test pits indicate the proposed infiltration facilities will generally be in the natural granular materials of Stratum B.

The Pennsylvania Protocol does not have any criteria for soil gradations or for the allowable percentage of fines (<#200 sieve) in the soil, but the other standards referenced in the Pennsylvania Manual limit the clay content to 20 percent by weight, and the combined silt/clay content to 40 percent by weight.

Soil laboratory testing of the subgrade material was performed in accordance with ASTM D-2487 and is included in Enclosure (3). The laboratory gradation and classification test results are summarized as follows:

Soil Classifications Summary

Test Pit	Soil Sample Depth/ <u>Elevation</u>	<u>Stratum</u>	Classification	% <u>Moisture</u>	Combined Silt/Clay (%<#200)
TP-I-1A	4.8' El 890.0	В	poorly graded GRAVEL with silt and sand (GP-GM), cobbles	4.4%	5%
TP-I-1B	16.2' El 890.0	В	poorly graded GRAVEL with sand (GP), cobbles	4.2%	2%
TP-I-2A	4.8' El 868.0	В	poorly graded GRAVEL with silt and sand (GP-GM), cobbles	6.3%	9%
TP-I-4B	9.7' El 897.0	В	poorly graded GRAVEL with sand (GP), cobbles	2.8%	2%
TP-I-5A	1.9' El 894.0	В	silty GRAVEL with sand (GM)	6.3%	15%
TP-I-6A	10.2' El 890.0	В	poorly graded SAND (SP)	5.3%	1%
TP-I-6C	1.5' El 890.0	В	poorly graded GRAVEL with silt (GP-GM)	5.2%	11%
TP-I-6E	0.5' El 888.0	F	silty sand with gravel – FILL (SM)	15.2%	27%
TP-15	4.0' El 907.7	В	poorly graded GRAVEL with sand (GP)	2.6%	1%

The proposed infiltration facilities are considered to be feasible in the natural granular material of Stratum B.

4.3 In-Situ Infiltration Rates

Protocol 2 recommends that soils underlying infiltration devices should have infiltration rates between 0.1 and 10 inches per hour.

In-situ infiltration testing was conducted at a depth of approximately 0.5 feet to 16.2 feet below existing grades as indicated on the site plan included in Enclosure (4). The test method referenced in Protocol 1, as summarized herein, was used to conduct the in-situ infiltration testing.

A test pit was excavated to the test depth at the study location. A solid 4-inch I.D. PVC casing was installed and seated approximately 2 inches into the underlying soil subgrades. The outer ring of the infiltration test casing was sealed with a water-bentonite soil mixture. The test location was presoaked for 1 hour with a 12-inch depth of water immediately prior to testing with the water level re-established at 30-minute intervals. The drop in the water level during the last 30 minutes of the presoak period was used to determine the time interval used for the infiltration test in accordance with Protocol 1.

Measurements were taken at the appropriate time interval for a total of 8 readings obtained or until a stabilized rate of drop was obtained, whichever occurred first. A stabilized rate of drop is defined by Protocol 1 as a difference of ¼-inch or less of drop between the highest and lowest readings of four consecutive readings. The water level in the infiltration test casing was re-established after each reading.

The final in-situ infiltration rate was calculated as the average stabilized rate or the drop in water level during the final time period, expressed as inches per hour. Infiltration rates listed as zero includes tests where the rate of infiltration was below a measurable rate, less than \$^{1}/_{16}\$ of an inch per hour.

The observed in-situ infiltration rates at the test locations and test depths are summarized below:

<u>Basin</u>	Test <u>Pit</u>	Estimated Surface Grade	Infiltration Test Grade	<u>Stratum</u>	Soil Type	Average In-situ Infiltration Rate (in/hr.)
1 (BOB = 890.0)	I-1A	El 894.8	El 890.0 (4.8')	В	poorly graded GRAVEL with silt and sand (GP-GM)	>10 in/hr.
	I-1B	El 906.2	El 890.0 (16.2')	В	poorly graded GRAVEL with sand (GP)	>10 in/hr.
	I-1C	El 889.0	El 888.5 (0.5')	В	silty SAND with gravel (SM)	9½ in/hr.
	I-1D	El 888.0	El 887.5 (0.5')	В	silty SAND with gravel (SM)	7½ in/hr.
2 (BOB = 868.0)	I-2A	El 872.8	El 868.0 (4.8')	В	poorly graded GRAVEL with silt and sand (GP-GM)	>10 in/hr.
	I-2B	El 871.3	El 868.0 (3.3')	В	poorly graded GRAVEL with sand (GP)	>10 in/hr.
4 (BOB =	I-4A	El 906.7	El 897.0 (9.7')	В	poorly graded SAND with gravel (SP)	>10 in/hr.
897.0)	I-4B	El 906.7	El 897.0 (9.7')	В	poorly graded GRAVEL with sand (GP)	>10 in/hr.
	I-4C	El 904.4	El 897.0 (7.4')	В	poorly graded GRAVEL with sand (GP)	>10 in/hr.
5 (BOB = 894.0)	I-5A	El 895.9	El 894.0 (1.9')	В	silty GRAVEL with sand (GM)	6 in/hr.
6 (BOB =	I-6A	El 900.2	El 890.0 (10.2')	В	poorly graded SAND (SP)	>10 in/hr.
890.0)	I-6B	El 900.1	El 890.0 (10.1')	В	poorly graded GRAVEL with sand (GP)	>10 in/hr.
	I-6C	El 891.5	El 890.0 (1.5')	В	poorly graded GRAVEL with silt (GP-GM)	>10 in/hr.
	I-6D	El 889.3	El 888.8 (0.5')	В	poorly graded GRAVEL with silt and sand (GP-GM)	>10 in/hr.
	I-6E	El 888.5	El 888.0 (0.5')	F	silty SAND with gravel (SM)	1¾ in/hr.

Note: BOB = Bottom of Basin

Based upon the recommended infiltration rates in Protocol 2, infiltration facilities are not considered feasible in the natural subgrade materials of Stratum B in Basins #1, #2, #4, and #6. These infiltration rates are considered to be excessive. Infiltration facilities are considered feasible in the natural materials of Stratum B in Basin #5.

4.4 Safety Factors

Protocol 2 recommends the minimum safety factor that may be used is two (2). It further recommends that a minimum safety factor of three (3) be used for soils which classify as silty loam, clay loam, silty clay loam, sandy clay loam or clay under the USDA classification system if the percolation test methodology is used.

Based on the gradation and classification of materials encountered, we recommend a safety factor of two (2) in the natural granular materials of Stratum B.

4.5 <u>Infiltration Design Recommendations</u>

Based on the measured in-situ rates at the proposed infiltration facility grades, we recommend the following in-situ infiltration design parameters:

			Recommended In-situ Infiltration	Recommended Design
<u>Area</u>	<u>Test Pit</u>	<u>Stratum</u>	Rate (in/hr.)	Factor of Safety
Basin #1	TP-I-1A, -1B, -1C, -1D	В	>10 in/hr.	2
Basin #2	TP-I-2A, -2B	В	>10 in/hr.	2
Basin #4	TP-I-4A, -4B, -4C	В	>10 in/hr.	2
Basin #5	TP-I-5A	В	6 in/hr.	2
Basin #6	TP-I-6A, -6B, -6C	В	>10 in/hr.	2

If infiltration facilities are incorporated into the development, we recommend that the infiltration facilities be designed in strict accordance with Pennsylvania Stormwater Best Management Practices most recent edition of the manual.

4.6 <u>Remediation – Subgrade Preparation Recommendations</u>

In order to prepare subgrade areas where infiltration rates are excessive, we recommend a subgrade augmentation program. An engineered soil may be used to modify subgrades which have excessive infiltration rates. This engineered soil buffer consists of a mixture of sand and topsoil/compost and may be used to develop a subgrade infiltration rate to within a range of 4 to 8 inches per hour and consider a factor of safety of two (2).

An estimated proposed subgrade cross-section for this construction process is included for reference as Figure No. 6-1 in Enclosure (6). The soil buffer should be tested after installation to ensure the required infiltration rates are achieved.

5. WATER QUALITY BASIN #3

Two test pits were excavated in Water Quality Basin #3 to determine the existing soil conditions. Test pits were excavated to depths of 16 to 17 feet. The existing soils classified as poorly graded gravel with sand and included cobbles. Infiltration testing was not performed but would be anticipated to be in excess of 10 inches per hour. Sealing of the basin bottom will require a liner or a layer of impervious material.

6. OBSERVATIONS REQUIREMENTS AND STUDY LIMITATIONS

The report is based on the design concept of the proposed project as furnished to our office during the preparation of this report. Any substantial changes in construction locations or grading should be brought to our attention so that we may determine any effect on the recommendations given herein.

The analysis and recommendations submitted in this report are based upon the test pit data and the site plans provided to us. This report does not reflect variations which may occur between the test locations. The nature and extent of variations between test locations may not become evident until the course of construction. It is recommended that on-site observation of facility installations be performed during the construction period to ascertain if re-evaluation of the recommendations of this report must be made.

We have prepared this report for the use of your office and the design professionals for design and planning purposes in accordance with generally accepted geotechnical engineering practices. No other warranties, either expressed or implied are made as to the professional services included in this report.

We appreciate the opportunity to be of service to you for this project. Please do not hesitate to contact us for further clarification of any aspect of this study.

Sincerely,

MIDLANTIC ENGINEERING, INC.

Timothy Burns, P.E.

President

Encls:

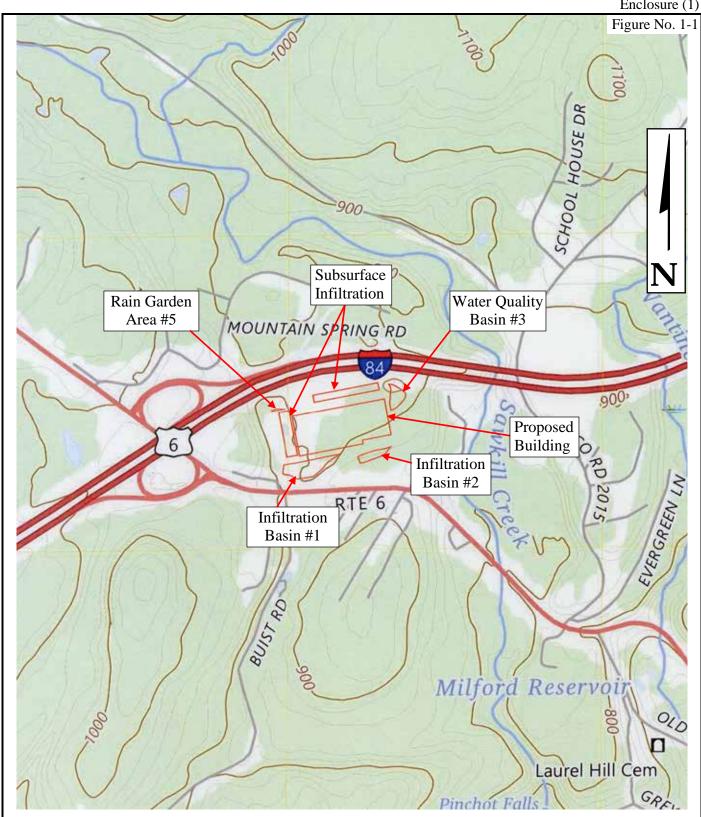
- (1) Project Vicinity Plans
 - Project Vicinity Plan USGS 2023, Figure No. 1-1
 - Project Vicinity Plan Aerial 2023, Figure No. 1-2
- (2) Geologic Mapping
 - SCS Mapping, Figure No. 2-1
 - Surficial Geologic Mapping, Figure No. 2-2
- (3) Laboratory Testing Data
 - Soil Classifications Summary
 - Gradation and Classifications (9 Sheets)
- (4) Subsurface Investigation Report
 - General Notes
 - Identification of Soils
 - Test Pit Location Plan, Figure No. 4-1
 - Test Pit Logs (TP-I-1A through TP-I-6E, TP-15 and TP-16) (17 sheets)
- (5) In-Situ Infiltration Testing Setup
- (6) Detail Prepared Infiltration Subgrade, Figure No. 6-1

cc: LVL Engineering Group

Attn: Mr. Kris J. Reiss, P.E. kreiss@lvlengineers.com

Attn: Mr. Michael E. Gable, P.E.

mgable@lvlengineers.com





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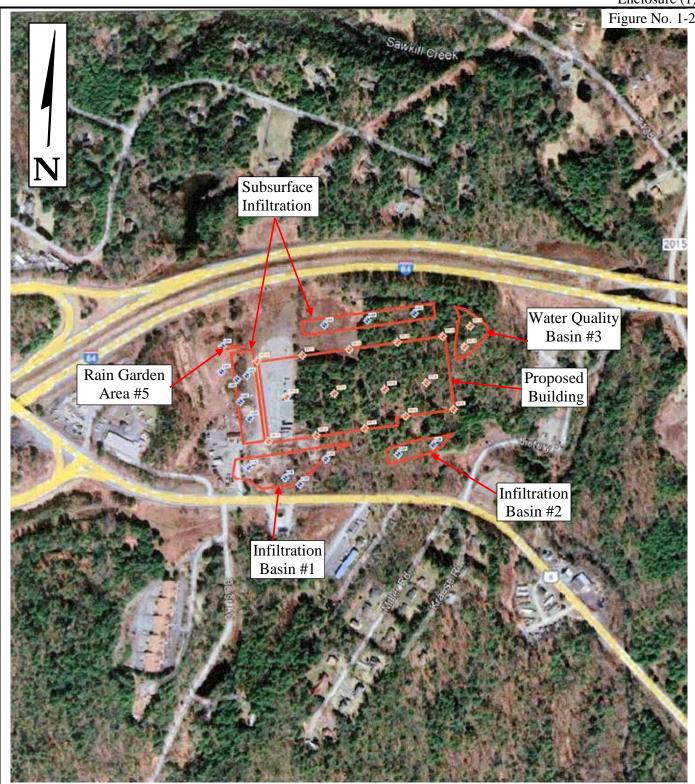
Drawing Title:

Project Vicinity Plan – USGS 2023

Milford Warehouse Development

Milford Township, PA

Drawn By:	Checked By:	Scale:
LJ	TB	1" = 1,000
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08/18/23	23004-SWM	





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Drawing Title:

Project Vicinity Plan – Aerial 2023

Milford Warehouse Development

Milford Township, PA

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Map Unit Legend

Symbol	USCS Classifications
89B – Chenango gravelly fine sandy loam 0" – 10" – gravelly fine sandy loam 10" – 29" – gravelly fine sandy loam, very gravelly fine sandy loam 29" – 70" – extremely cobbly loamy coarse sand, extremely gravelly loamy coarse sand 9B – Craigsville – Wyoming complex	SM SM, GM GM, GP-GM, GW-GM
0" – 5" – gravelly sandy loam	SM, SC
5" – 27" – gravelly, sandy loam, cobbly loam	SM, GM
27" – 60" – very gravelly loamy sand, very gravelly sandy loam, very cobbly loamy sand	GM, GP-GM



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Drawing Title:

SCS Mapping

Milford Warehouse Development Milford Township, PA

200

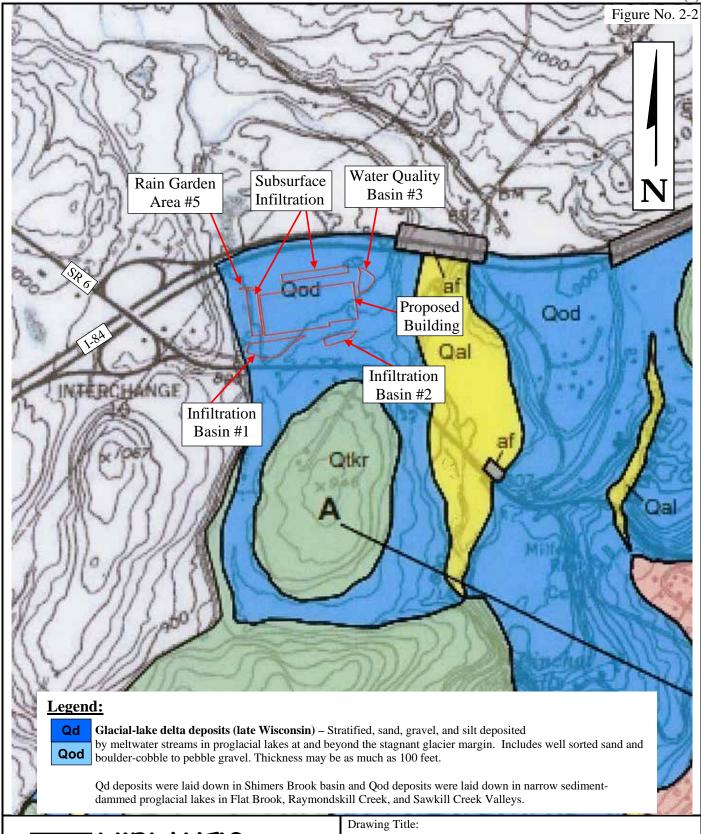
0 150 300 600 900

Map projection: Web Mercator Corner coordinates: WGS84 Edge tics: UTM Zone 18N WGS84

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⇒Meters 300

⇒Feet





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Surficial Geology Mapping

Milford Warehouse Development

Milford Township, PA

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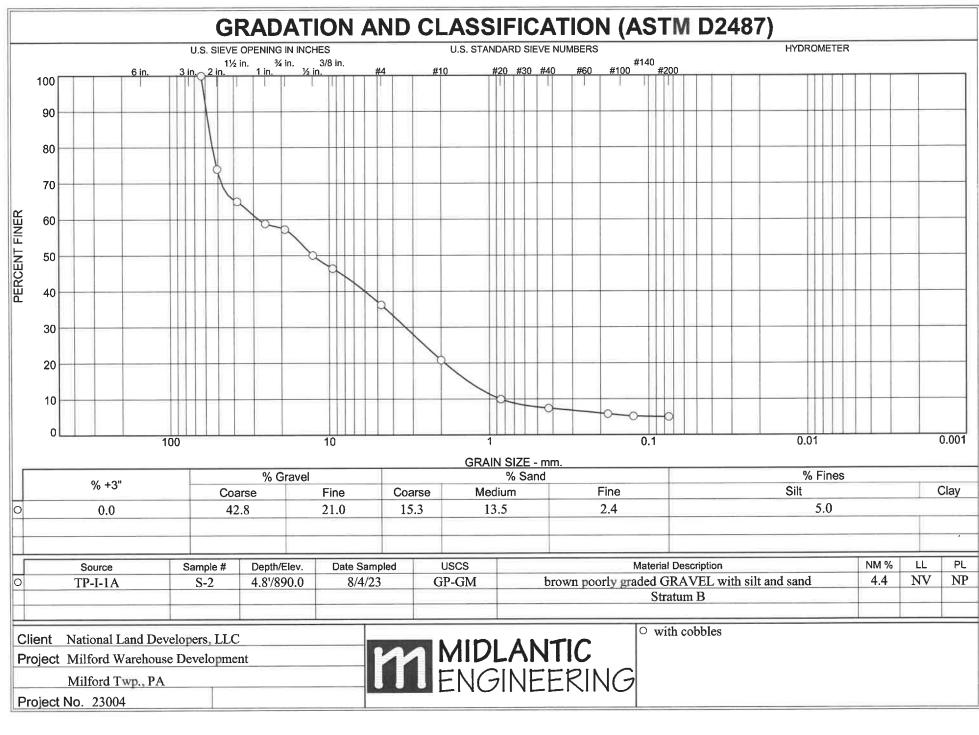
LABORATORY TEST DATA

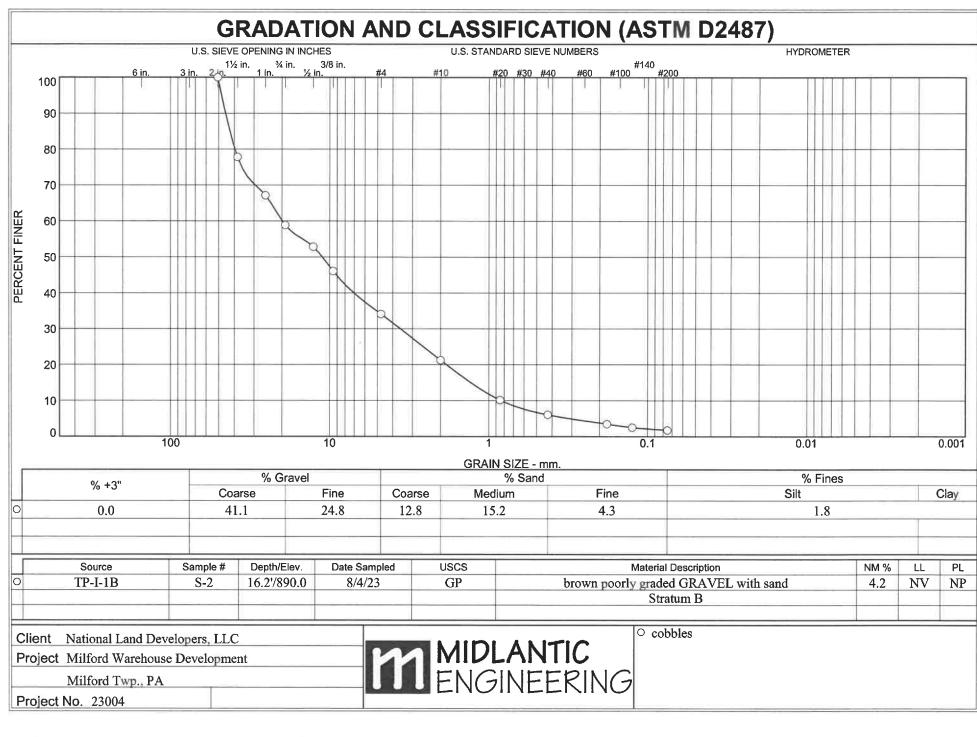
Soil Classifications SummaryGradation and Classifications (9 Sheets)

SOIL CLASSIFICATIONS SUMMARY

Results of testing are summarized in the following table, and the individual gradation and classification curves are included within this enclosure.

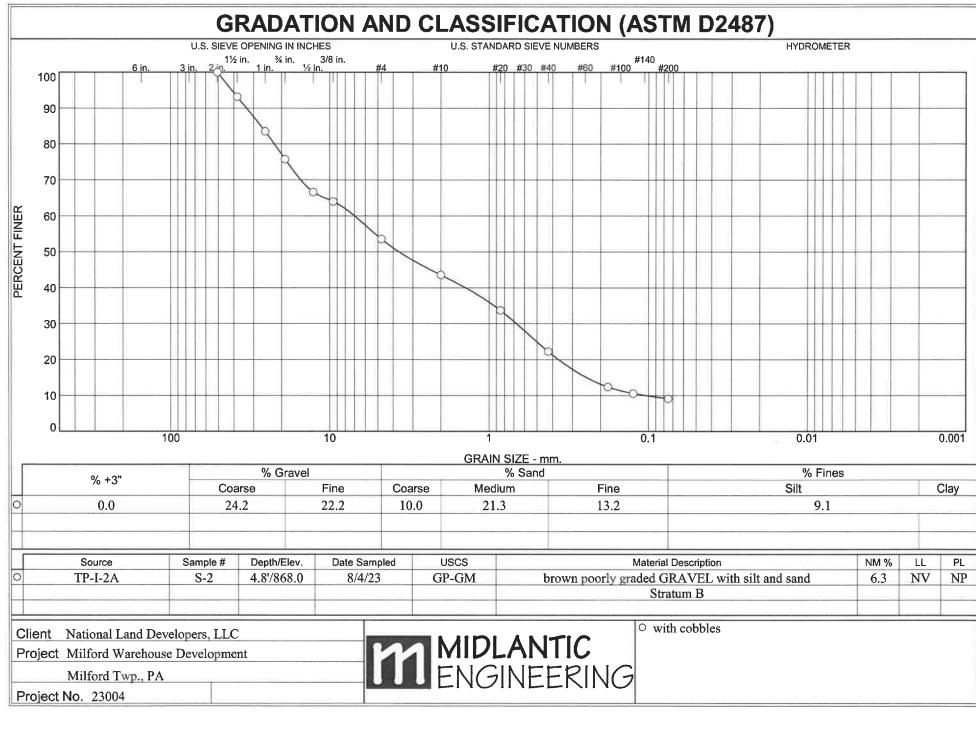
Test Pit	Soil Sample Depth/ <u>Elevation</u>	<u>Stratum</u>	Classification	% <u>Moisture</u>	Combined Silt/Clay (%<#200)
TP-I-1A	4.8' El 890.0	В	poorly graded GRAVEL with silt and sand (GP-GM), cobbles	4.4%	5%
TP-I-1B	16.2' El 890.0	В	poorly graded GRAVEL with sand (GP), cobbles	4.2%	2%
TP-I-2A	4.8' El 868.0	В	poorly graded GRAVEL with silt and sand (GP-GM), cobbles	6.3%	9%
TP-I-4B	9.7' El 897.0	В	poorly graded GRAVEL with sand (GP), cobbles	2.8%	2%
TP-I-5A	1.9' El 894.0	В	silty GRAVEL with sand (GM)	6.3%	15%
TP-I-6A	10.2' El 890.0	В	poorly graded SAND (SP)	5.3%	1%
TP-I-6C	1.5' El 890.0	В	poorly graded GRAVEL with silt (GP-GM)	5.2%	11%
TP-I-6E	0.5' El 888.0	F	silty sand with gravel – FILL (SM)	15.2%	27%
TP-15	4.0' El 907.7	В	poorly graded GRAVEL with sand (GP)	2.6%	1%

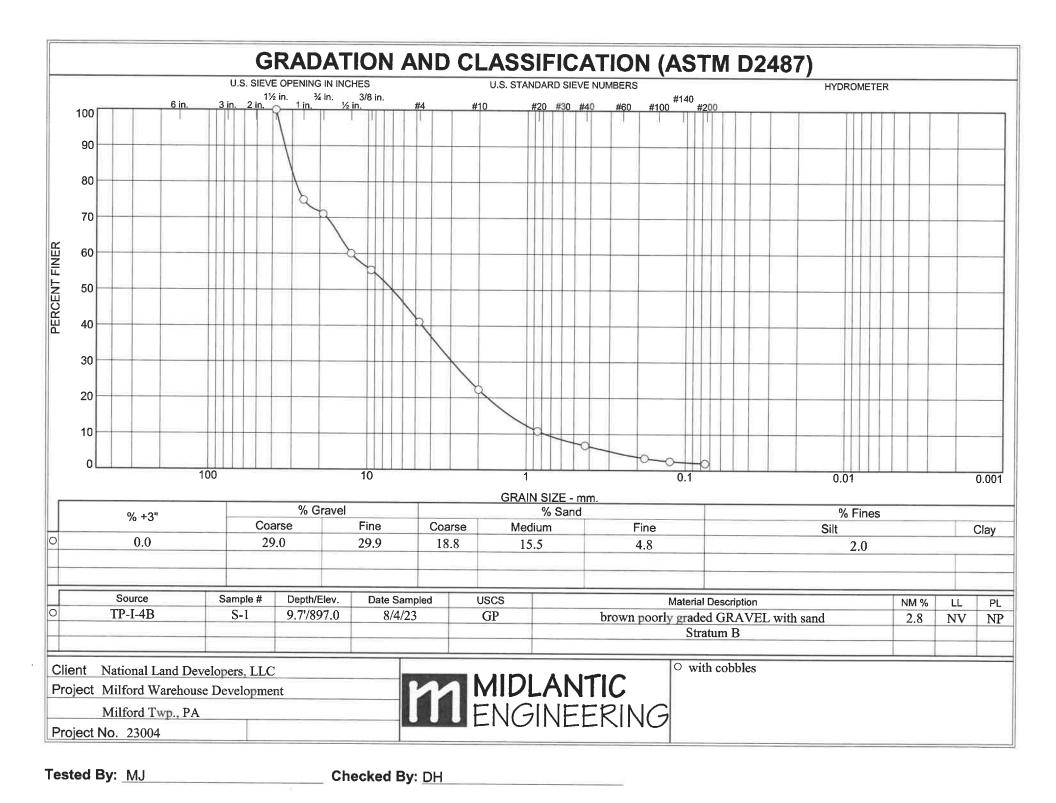


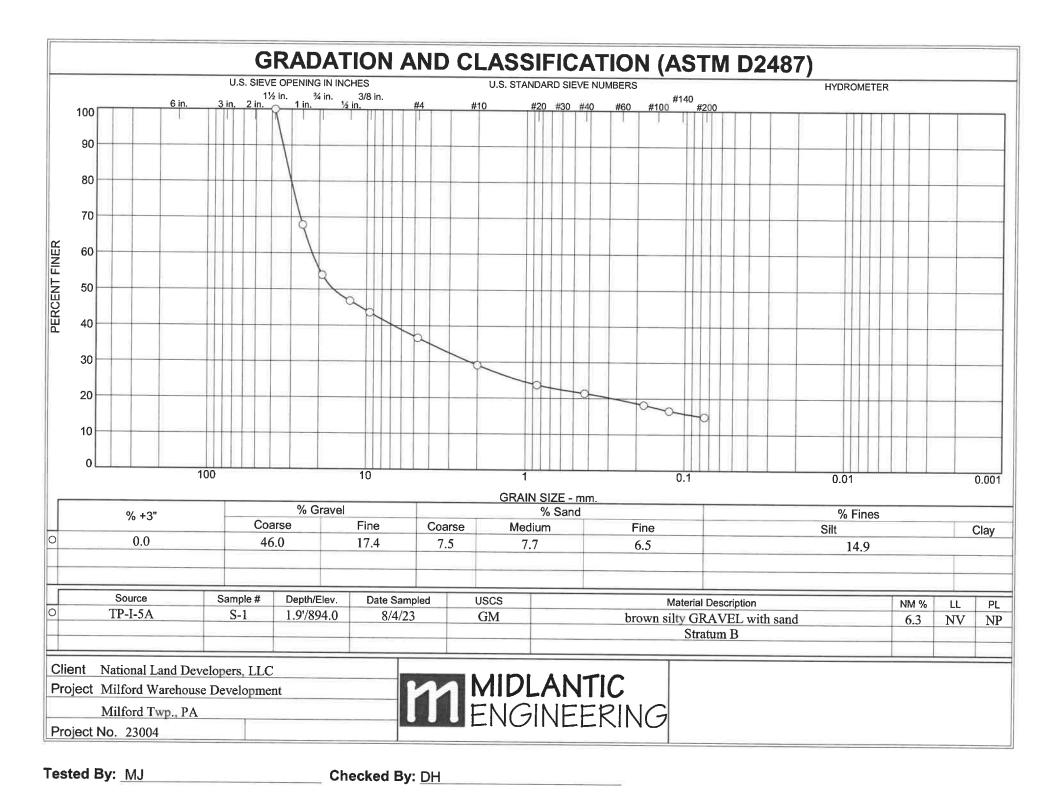


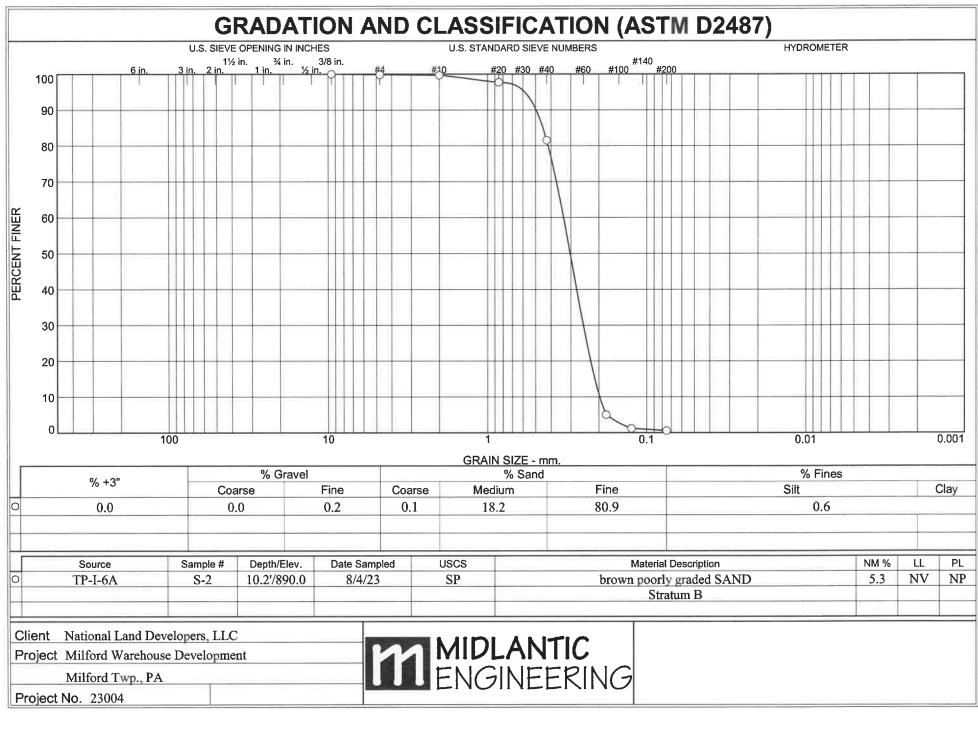
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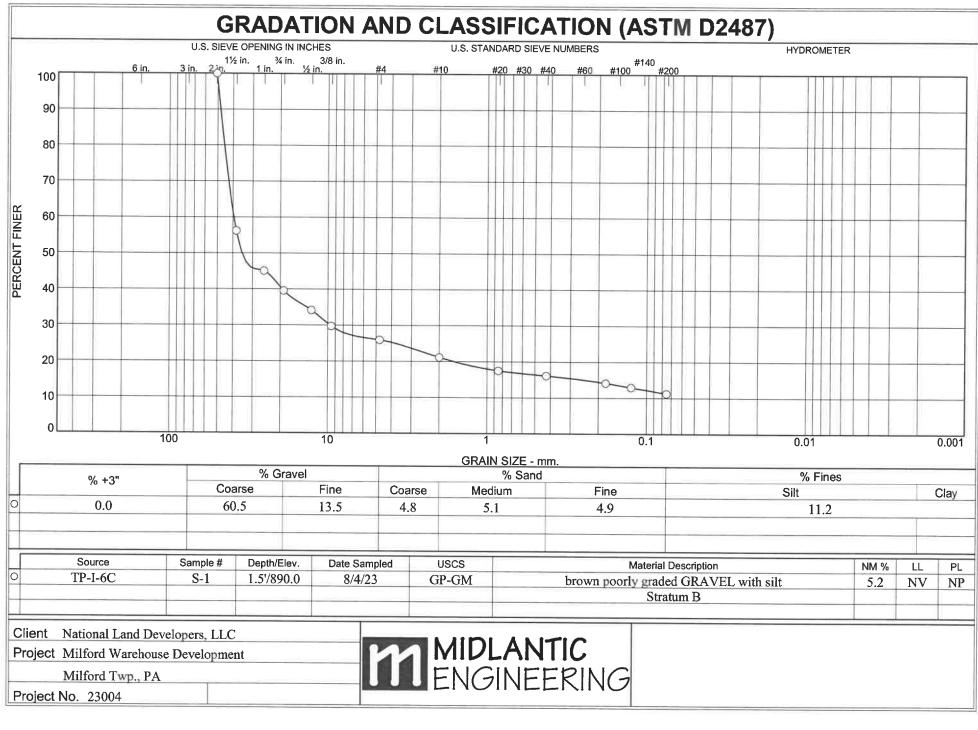
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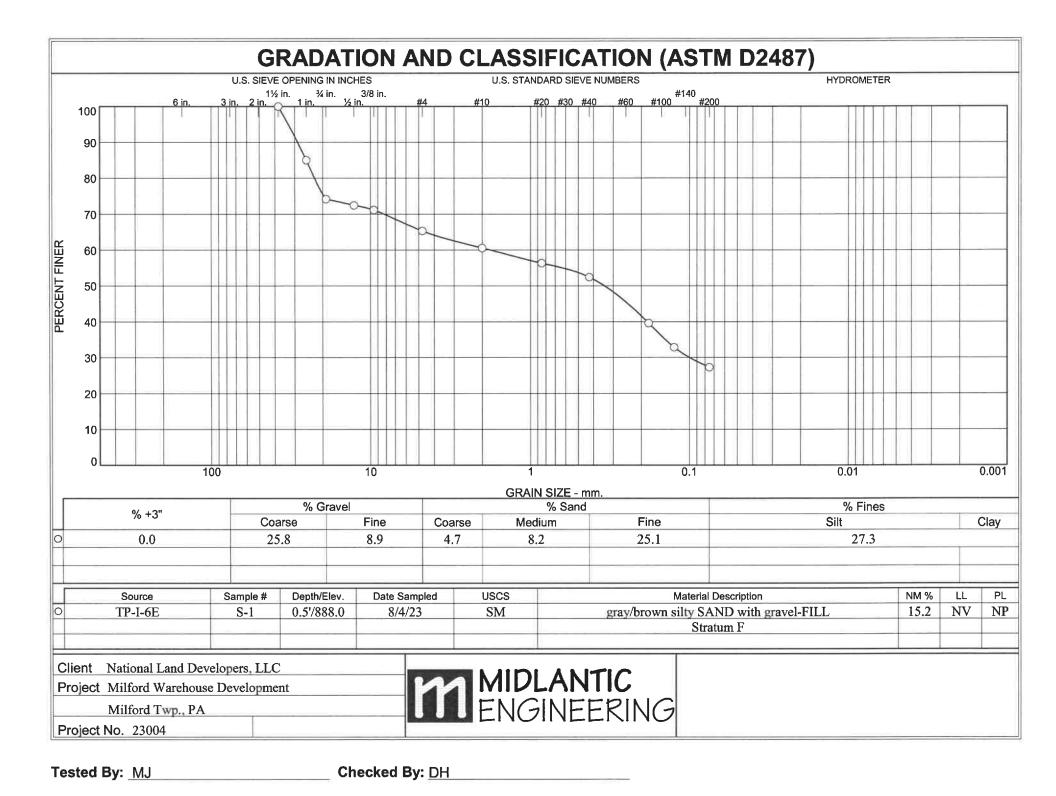


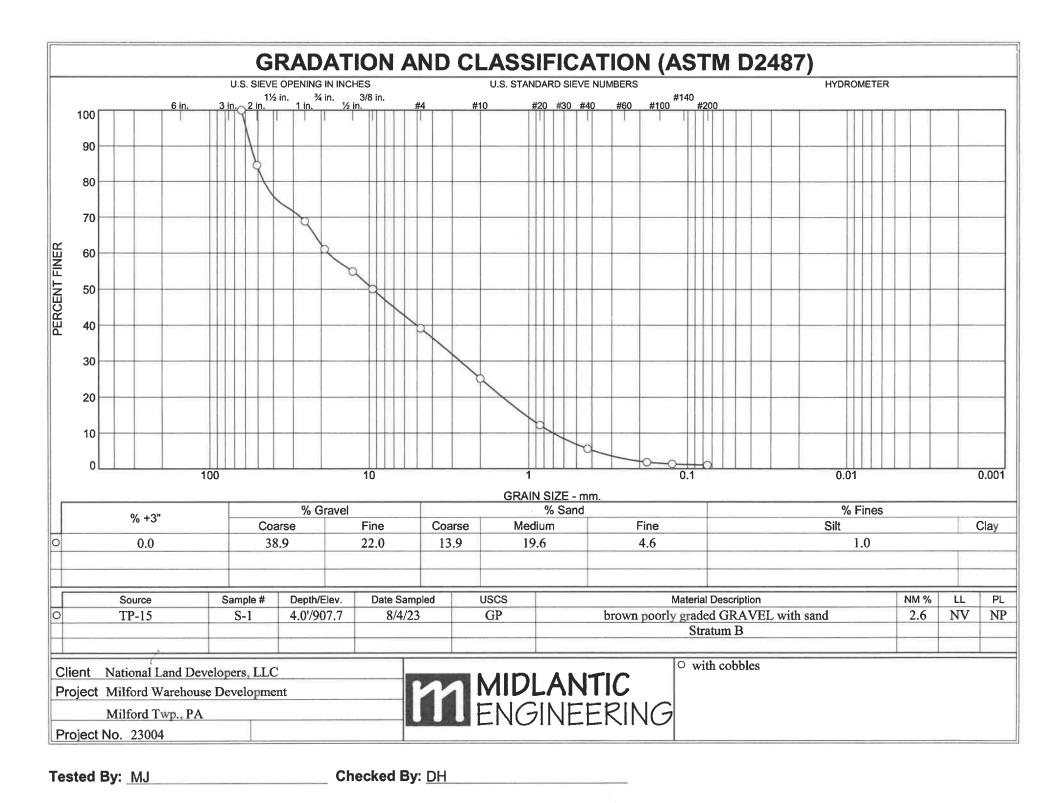










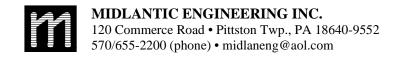


SUBSURFACE INVESTIGATION REPORT

General Notes
Identification of Soils
Test Pit Location Plan, Figure No. 4-1
Test Pit Logs (TP-I-1A through TP-I-6E, TP-15 and TP-16) (17 sheets)

GENERAL NOTES

- 1. Test pits are logged by engineering personnel to provide a record for geotechnical evaluation. The log itself includes a description of soil and rock materials encountered using visual classification in the field. Boundary lines between various strata are identified where possible and a graphical presentation is included based on the material excavated from the pit. Any significant features, such as fill conditions, underground structures, groundwater or water seepage conditions are recorded.
- 2. The test pit logs and related information depict subsurface conditions only at the specific location and at the particular time excavated. Soil conditions at other locations may differ from conditions occurring at these test pit locations. Also, the passage of time may result in a change in the subsurface soil and groundwater conditions at these locations.
- 3. The stratification lines represent the approximate boundary between soil and rock types as observed in the test pits. The profiles and water level observations presented have been made with reasonable care and accuracy and must be considered only an approximate representation of subsurface conditions to be encountered at the particular location.
- 4. Strata descriptions are based on visual inspection and are in accordance with the Unified Soil Classification System, per ASTM D-2488.
- 5. The test pit locations and grades are based on information provided by others and should be considered as approximate only. The test pit observations and draft logs were prepared by Mr. Daniel Hartigan, P.E. of this office.



IDENTIFICATION OF SOILS

I. Definition of Soil Group Names – ASTM D-2487-11				Group Name
	Gravels – More than 50% of coarse	Clean Gravels	GW	well-graded gravel
	fraction retained on No. 4 sieve	Less than 5% fines	GP	poorly graded gravel
Coarse-Grained Soils	Coarse, 34" to 3" Fine, No. 4 to 34"	Gravels with Fines	GM	silty gravel
More than 50%	Coarse, 74 to 3 Fine, No. 4 to 74	More than 12% fines	GC	clayey gravel
retained on	Sands – 50% or more of coarse	Clean Sands	SW	well-graded sand
No. 200 sieve	fraction passes No. 4 sieve	Less than 5% fines	SP	poorly graded sand
110. 200 sieve	Coarse: No. 10 to No. 4 Medium: No. 40 to No. 10 Fine: No. 200 to No. 40	Sands with Fines	SM	silty sand
		More than 12% fines	SC	clayey sand
	Silts and Clays – Liquid Limit less than 50 Low to medium plasticity	Inorganic	CL	lean clay
			ML	silt
Eine Conine I Ceile		Organic	OL	organic clay
Fine-Grained Soils			OL	organic silt
50% or more passes the No. 200 sieve		т	CH	fat clay
the No. 200 sieve	Silts and Clays – Liquid Limit 50 or	Inorganic	MH	elastic silt
	more Medium to high plasticity	0	ОН	organic clay
		Organic	OH	organic silt
Highly Organic Soils	Primarily organic matter, dark in color, a	and organic odor	PT	peat

II. Definition of Minor C	omponent Proportions	Approximate Percentage of Fraction by Weight
adjective form	gravelly, sandy	30% or more coarse grained
:t-la	sand, gravel	15% or more coarse grained
with	silt, clay	5% to 12% fine grained
4	sand, gravel	Less than 15% coarse grained
trace	silt, clay	Less than 5% fine grained

III. Glossary of Miscellaneous Terms

symbols Unified Soil Classification Symbols are shown above as group symbols. Use a Line Chart for laboratory identification.

Dual symbols are used for borderline classifications.

boulders & cobbles Boulders are considered rounded pieces of rock larger than 12 inches, while cobbles range from 3 to 12 inch size.

disintegrated rock Very generally defined as residual rock material with a standard penetration resistance (SPT) of more than 50 blows per

foot, and less than refusal. Refusal is defined as a SPT of 100 blows for 2" or less penetration.

rock fragments Angular pieces of rock, distinguished from transported gravel, which have separated from original vein or strata and are

present in a soil matrix.

quartz A hard silica mineral often found in residual soils

ironite Iron oxide deposited within a soil layer forming cemented deposits

cemented sand Usually localized rock-like deposits within a soil stratum composed of sand grains cemented by calcium carbonate or

other materials.

mica A soft plate of silica mineral found in many rocks, and in residual or transported soil derived therefrom.

organic materials Topsoil: Surface soils that support plant life and which contain considerable amounts of organic matter;

(excluding peat) Organic Matter: Soil containing organic colloids throughout its structure;

Lignite: Hard, brittle decomposed organic matter with low fixed carbon content (a low grade of coal).

fill Man made deposit containing soil, rock and often foreign matter

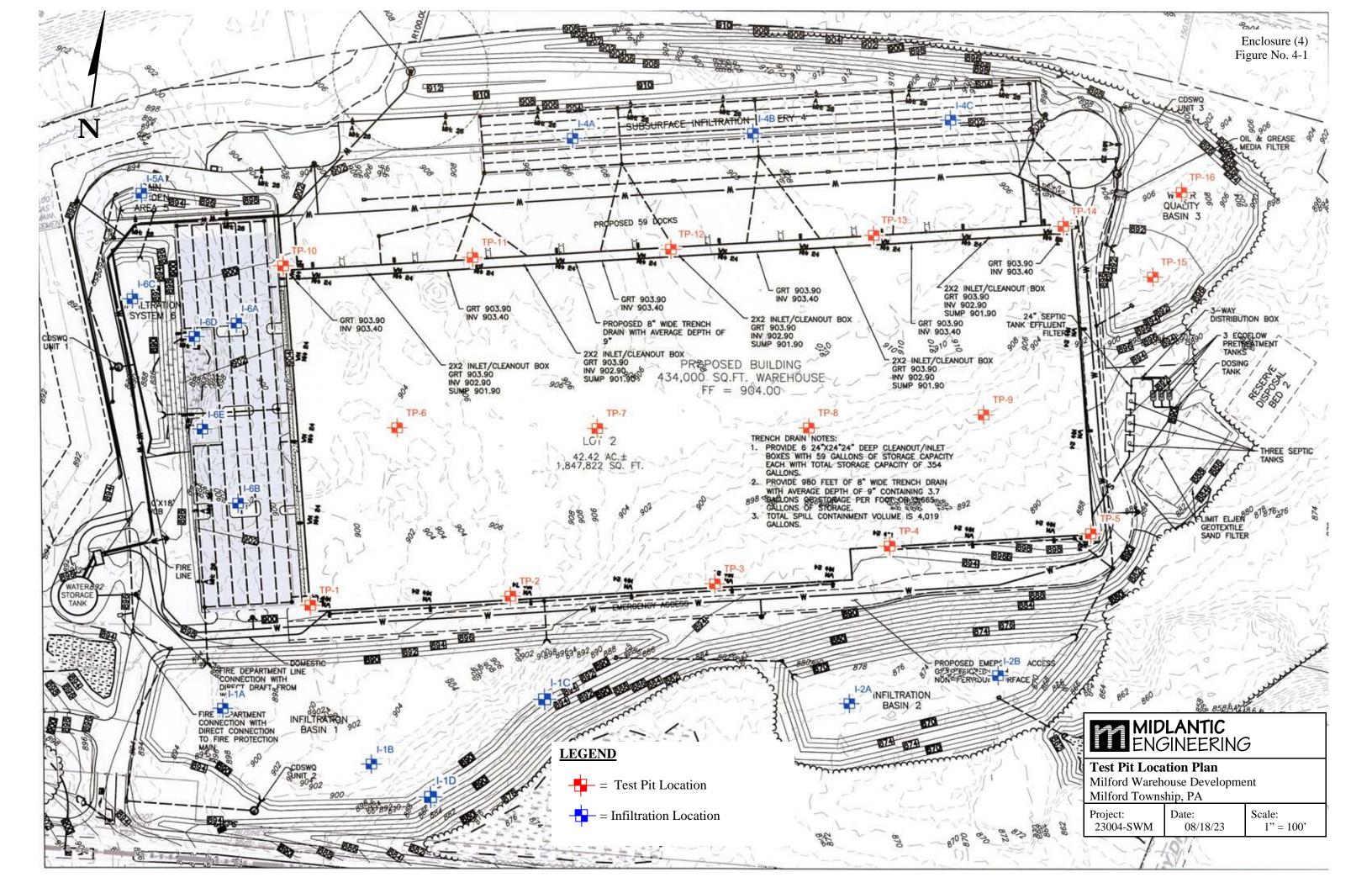
probable fill Soils which contain no visually detected foreign matter but which are suspect with regard to origin

 lenses
 0 to 2 inch seam of minor soil component

 layers
 2 to 12 inch seam of minor soil component

 pocket
 Discontinuous body of minor soil component

color shadesLight to dark to indicate substantial difference in colormoisture conditionsWet, moist, or dry to indicate visual appearance of specimen





TEST PIT LOG TP-I-1A

Project: Milford Warehouse Development

Milford Township, PA

Test Loc. No. TP-I-1A

Contract No. 23004

Date Excavated: 08/03/23

ME, Inc. Rep.: DH

Equip. Used: Excavator

Surface Elev.: 894.8

Groundwater Observations

Encountered:	08/03/23	Depth:	none
Completion:	08/03/23	Depth:	dry



			20 4 345	100 May 200	The state of the s	THE PART OF THE PARTY	W. W. Lindson	地国的国际企业
Depth (ft.)	Strata Description	Class.	Str'm	Elev.	InS Depth	itu Testing Geo. Pen.	M (%)	Remarks
2—	7" topsoil brown poorly graded GRAVEL with silt and sand, cobbles	GP- GM	В		2 —		9.4	
4					4		4.4	infiltration testing @ 4.8'; El 890.0
6				887.8	6 —			
8	Bottom of Test Pit at 7.0 feet				8 —			
10 —					10 —			
12-					12 —			
14					14 —			
16—					16 —			
18					18			
20 —					12 —			

MIDLANTIC ENGINEERING

TEST PIT LOG TP-I-1B

Project: Milford Warehouse Development

TP-I-1B

Milford Township, PA

Contract No. 23004

Test Loc. No.

Date Excavated: 08/07/23

ME, Inc. Rep.: DH

Equip. Used: Excavator

Surface Elev.: 906.2

Groundwater Observations

Encountered:	08/07/23	Depth:	none	
Completion:	08/07/23	Depth:	dry	



Depth (ft.)	Strata Description	Class.	Str'm	Elev.	InS Depth	itu Testing Geo. Pen.	M (%)	Remarks
(11.)	3" topsoil, brick, stone – FILL		F		Depth	Geo. Pen.		
	1							
2				904.2	2 —			
2					2 -			
	†							
4	brown poorly graded GRAVEL with silt and sand	GP- GM	В		4			
	with siit and said	GWI						
6 —					6 —			
	-							
8					8 —			
Ü					Ü		5.2	
	1						0.2	
10 —	-				10 —			
	-							
10					10			
12 —	brown poorly graded GRAVEL	GP			12			
	with sand, occasional cobbles							
14	4				14			
16	1				16 —		4.2	— infiltration testing
	4							@ 16.2'; El 890.0
18					10			
16	Bottom of Test Pit at 18.5 feet			887.7	18			
	Douloin of Test Pit at 18.5 feet							
20 —					12 —			



TEST PIT LOG TP-I-1C

Project: Milford Warehouse Development

Milford Township, PA

Test Loc. No. **TP-I-1C** (relocated)

Contract No. 23004

Date Excavated: 08/03/23

ME, Inc. Rep.: DH

Equip. Used: Excavator

Surface Elev.: 889.0

Groundwater Observations

Encountered:	08/03/23	Depth:	none	
Completion:	08/03/23	Depth:	dry	



			100	- Classical	The second		-w 2 2	AND THE PERSON NAMED IN
Depth (ft.)	Strata Description	Class.	Str'm	Elev.	InS Depth	itu Testing Geo. Pen.	M (%)	Remarks
	5" topsoil						10.5	infiltration testing
								@ 0.5'; El 888.5
	yellow, brown silty SAND with	SM	В					
2	gravel				2 —			
-	1							
4				885.0	4			
4	Bottom of Test Pit at 4.0 feet				4 ——			
	-							
6					6 —			
8					8 —			
	-							
10					10 —			
10					10			
12					12 —			
14					14 —			
16 —					16 —			
	-							
10					10			
18 —]				18 —			
	-							
20 —					12 —			
20 -					12			



TEST PIT LOG TP-I-1D

Project: Milford Warehouse Development

Milford Township, PA

Test Loc. No. TP-I-1D (added)

Contract No. 23004

Date Excavated: 08/03/23

ME, Inc. Rep.: DH

Equip. Used: Excavator

Surface Elev.: 888.0

Groundwater Observations

Encountered:	08/03/23	Depth:	none	
Completion:	08/03/23	Depth:	drv	



Depth (ft.)	Strata Description	Class.	Str'm	Elev.	InS: Depth	itu Testing Geo. Pen.	M (%)	Remarks
(11.)	5" topsoil				Depth	Geo. Pen.	16.4	infiltration testing
	yellow, brown silty SAND with	SM	В					@ 0.5'; El 887.5
2	gravel				2 —			
,				884.0	4			
4	Bottom of Test Pit at 4.0 feet				4 —			
6 —					6 —			
8					8 —			
1.0					1.0			
10 —					10 —			
12					12 —			
14					14			
16-					16			
18					18			
20 —					12 —			



TEST PIT LOG TP-I-2A

Project: Milford Warehouse Development

Milford Township, PA

Test Loc. No. TP-I-2A

Contract No. 23004

Date Excavated: 08/03/23

ME, Inc. Rep.: DH

Equip. Used: Excavator

Surface Elev.: 872.8

Groundwater Observations

Encountered:	08/03/23	Depth:	none	
Completion:	08/03/23	Depth:	dry	



			Que.	ST 500 10 10	NAME OF TAXABLE PARTY.	THE RESERVE OF THE PARTY OF THE	0.000	THE CAN PERSON OF THE PERSON O
Depth (ft.)	Strata Description	Class.	Str'm	Elev.	InS Depth	itu Testing Geo. Pen.	M (%)	Remarks
2	3" topsoil brown poorly graded SAND with silt	SP- SM	В		2 —		12.6	
4	brown poorly graded GRAVEL with silt and sand, cobbles	GP- GM			4 —			
6	dark brown poorly graded SAND with silt and gravel	SP- SM		865.8	6 —		6.3 3.8	infiltration testing @ 4.8'; El 868.0
8	Bottom of Test Pit at 7.0 feet				8 —			
10 —					10 —			
12					12 —			
14					14			
16					16 —			
18					18 —			
20 —					12 —			



TEST PIT LOG TP-I-2B

Project: Milford Warehouse Development Milford Township, PA

Test Loc. No. TP-I-2B

Contract No. 23004

Date Excavated: 08/03/23

ME, Inc. Rep.: DH

Equip. Used: Excavator

Surface Elev.: 871.3

Groundwater Observations

Encountered:	08/03/23	Depth:	none	
Completion:	08/03/23	Depth:	dry	



Depth		a1			InS	itu Testing		
(ft.)	Strata Description	Class.	Str'm	Elev.	Depth	Geo. Pen.	M (%)	Remarks
	3" topsoil							
	brown silty sand – FILL	SM	F					
2	6" topsoil			869.3	2 —			
	o topson							
	harren ann de de CDAVEL estab	GP	D				9.5	infiltration testing @ 3.3'; El 868.0
4	brown poorly graded GRAVEL with sand, cobbles	GP	В		4			@ 3.3 ; El 808.0
_				865.3				
6 —	Bottom of Test Pit at 6.0 feet				6 —			
8					8			
O					o l			
10 —					10 —			
12					12			
14 ——					14			
16					16			
10					10			
18					18			
20 —					12 —			



TEST PIT LOG TP-I-4A

Project: Milford Warehouse Development

Milford Township, PA

Test Loc. No. TP-I-4A

Contract No. 23004

Date Excavated: 08/02/23

ME, Inc. Rep.: DH

Equip. Used: Excavator

Surface Elev.: 906.7

Groundwater Observations

Encountered:	08/02/23	Depth:	none	
Completion:	08/02/23	Depth:	dry	



			100 5	20		THE ASSESSMENT	ALC: UNK	《四周报法》,"全国国际公司
Depth (ft.)	Strata Description	Class.	Str'm	Elev.	InS Depth	itu Testing Geo. Pen.	M (%)	Remarks
	2" topsoil gray poorly graded gravel with sand – FILL	GP	F	905.2				
2 —	brown poorly graded SAND with gravel, cobbles	SP	В		2 —			
4	graver, coopies				4			
6					6 —			
8					8 —			
10					10 —		3.5	— infiltration testing
								@ 9.7'; El 897.0
12	Bottom of Test Pit at 12.0 feet			894.7	12 —			
14 —					14 —			
16					16			
18					18 —			
20 —	-	•	•	•	12 —			



TEST PIT LOG TP-I-4B

Project: Milford Warehouse Development Milford Township, PA

Test Loc. No. TP-I-4B

Contract No. 23004

Date Excavated: 08/02/23

ME, Inc. Rep.: DH

Equip. Used: Excavator

Surface Elev.: 906.7

Groundwater Observations

Encountered:	08/02/23	Depth:	none	
Completion:	08/02/23	Depth:	dry	



				Will San San St.	and the second	No. World to contrast	ACT TO THE	
Depth (ft.)	Strata Description	Class.	Str'm	Elev.	InS Depth	itu Testing Geo. Pen.	M (%)	Remarks
(ft.) 2	Strata Description 6" topsoil brown poorly graded GRAVEL with sand, cobbles	GP GP	Str'm	Elev.	Depth 2 4 6 8	Geo. Pen.	M (%)	Remarks
10 — 12 — 14 — 16 — 18 — —	Bottom of Test Pit at 12.0 feet			894.7	10 — 12 — 14 — 16 — 18 — — 18 — —		2.8	— infiltration testing @ 9.7' El 897.0
20 —		l			12 —		l	<u> </u>



TEST PIT LOG TP-I-4C

Project: Milford Warehouse Development

Milford Township, PA

Test Loc. No. TP-I-4C

Contract No. 23004

Date Excavated: 08/03/23

ME, Inc. Rep.: DH

Equip. Used: Excavator

Surface Elev.: 904.4

Groundwater Observations

Encountered:	08/03/23	Depth:	none	
Completion:	08/03/23	Depth:	dry	



Depth (ft.)	Strata Description	Class.	Str'm	Elev.	InSi Depth	itu Testing Geo. Pen.	M (%)	Remarks
(11.)	4" topsoil				Бери	Geo. 1 cm.		
	orange, brown poorly graded SAND with silt and gravel	SP- SM	В				11.0	
2 —	with sift and graver	SIVI			2 —		11.0	
4					4			
6	brown poorly graded GRAVEL with sand, cobbles	GP			6 —			
							2.9 -	— infiltration testing
8					8 —			— infiltration testing @ 7.4'; El 897.0
10	Bottom of Test Pit at 10.0 feet			894.4	10			
	bottom of Test Pit at 10.0 feet							
12-					12 —			
14					14			
14					14			
1.					16			
16					16			
-								
18					18			
20 —					12 —			



TEST PIT LOG TP-I-5A

Project: Milford Warehouse Development Milford Township, PA

Test Loc. No. TP-I-5A

Contract No. 23004

Date Excavated: 08/02/23

ME, Inc. Rep.: DH

Equip. Used: Excavator

Surface Elev.: 895.9

Groundwater Observations

Encountered:	08/02/23	Depth:	none	
Completion:	08/02/23	Depth:	dry	



Depth	Strata Description	Class.	Str'm	Elev.	InS	itu Testing	M (%)	Remarks
(ft.)	6" topsoil				Depth	Geo. Pen.	. /	
		C) I	D					
2	brown silty GRAVEL with sand	GM	В		2 —		6.3	infiltration testing
2					2			@ 1.9'; El 894.0
4					4			
	D. C.			890.9				
6	Bottom of Test Pit at 5.0 feet				6 —			
0					0			
8					8 —			
10					10 —			
10					10			
-								
12					12 —			
14					14			
14					14			
-								
16					16 —			
18					18 —			
18					18			
20 —					12 —			



TEST PIT LOG TP-I-6A

Project: Milford Warehouse Development

Milford Township, PA

Test Loc. No. TP-I-6A

Contract No. 23004

Date Excavated: 08/02/23

ME, Inc. Rep.: DH

Equip. Used: Excavator

Surface Elev.: 900.2

Groundwater Observations

Encountered:	08/02/23	Depth:	none	
Completion:	08/02/23	Depth:	dry	



			- TOTAL	Mary Million	WEST OF STREET		The state of the s	
Depth (ft.)	Strata Description	Class.	Str'm	Elev.	InS Depth	itu Testing Geo. Pen.	M (%)	Remarks
	3" topsoil							
	brown poorly graded GRAVEL with	GP	В					
2	sand, cobbles				2 —			
2					2			
4					4			
							3.7	
					,			
6					6 —			
8					8 —			
					O			
10 —	brown poorly graded SAND	SP			10 —		5.3	infiltration testing
	blowii poorty graded SAND	51					3.3	infiltration testing @ 10.2'; El 890.0
12 —				887.7	12 —			
	Bottom of Test Pit at 12.5 feet							
1.4					1.4			
14					14			
16					16 —			
					-			
18					18			
20 —	1	I	I		12 —		I	I



TEST PIT LOG TP-I-6B

Project: Milford Warehouse Development

Milford Township, PA

Test Loc. No. TP-I-6B

Contract No. 23004

Date Excavated: 08/02/23

ME, Inc. Rep.: DH

Equip. Used: Excavator

Surface Elev.: 900.1

Groundwater Observations

Encountered:	08/02/23	Depth:	none	
Completion:	08/02/23	Depth:	dry	



	T			of all and Miles are	たを整体の位すり			San Like Control of the Control of t
Depth (ft.)	Strata Description	Class.	Str'm	Elev.	InS Depth	itu Testing Geo. Pen.	M (%)	Remarks
	3" topsoil / gravel – FILL				-			
	brown poorly graded GRAVEL with	GP	В					
2	sand, cobbles	Gi			2 —			
2					2			
4					4			
4					4		4.4	
							4.4	
6 —					6 —			
Ü					Ü			
-								
8	-				8 —			
10 —					10 —		4.4	— infiltration testing
							4.4	@ 10.1'; El 890.0
12 —				887.6	12 —			
	Bottom of Test Pit at 12.5 feet			007.0				
14					14 —			
16 —					16 —			
18 —					18 —			
20 —	1	I	I		12 —	I	1	ı



TEST PIT LOG TP-I-6C

Project: Milford Warehouse Development Milford Township, PA

Test Loc. No. TP-I-6C

Contract No. 23004

Date Excavated: 08/02/23

ME, Inc. Rep.: DH

Equip. Used: Excavator

Surface Elev.: 891.5

Groundwater Observations

Encountered:	08/02/23	Depth:	none	
Completion:	08/02/23	Depth:	dry	



Depth (ft.)	Strata Description	Class.	Str'm	Elev.	InS Depth	itu Testing Geo. Pen.	M (%)	Remarks
(10.)	5" topsoil				Бериг	Geo. 1 cm.		
2 —	brown poorly graded GRAVEL with silt	GP- GM	В		2 —		5.2	infiltration testing @ 1.5'; El 890.0
4	Bottom of Test Pit at 4.0 feet			887.5	4			
	Bottom of Test Pit at 4.0 feet							
6					6 —			
0					U			
8 —					8 —			
10					10 —			
12					12			
14					14			
16					16 —			
18					18 —			
20 —					12 —			



TEST PIT LOG TP-I-6D

Project: Milford Warehouse Development

Milford Township, PA

Test Loc. No. TP-I-6D

Contract No. 23004

Date Excavated: 08/02/23

ME, Inc. Rep.: DH

Equip. Used: Excavator

Surface Elev.: 889.3

Groundwater Observations

Encountered:	08/02/23	Depth:	none	
Completion:	08/02/23	Depth:	dry	



			1000		The state of the s	- Parkey your moderate	1 Starten Contract	題などの国際は国際
Depth (ft.)	Strata Description	Class.	Str'm	Elev.	InS Depth	itu Testing Geo. Pen.	M (%)	Remarks
	5" topsoil						20.7	infiltration testing
		CD	D					@ 0.5'; El 888.8
	gray poorly graded GRAVEL with silt and sand	GP- GM	В					
2	sitt and said	Givi			2 —			
	brown, orange poorly graded SAND with gravel	SP						
4					4		14.3	
4					4			
	Bottom of Test Pit at 5.0 feet			884.3				
	Bottom of Test Pit at 5.0 feet							
6 —					6 —			
8					8 —			
10 —					10 —			
12					12 —			
14					14			
16					16 —			
18					18 —			
20 —					12 —			



TEST PIT LOG TP-I-6E

Project: Milford Warehouse Development

Milford Township, PA

Test Loc. No. TP-I-6D

Contract No. 23004

Date Excavated: 08/02/23

ME, Inc. Rep.: DH

Equip. Used: Excavator

Surface Elev.: 888.5

Groundwater Observations

Encountered:	08/02/23	Depth:	3.0'
Completion:	08/02/23	Depth:	3.0'



	1	1			Ball A	AMERICAN STREET, CO.	The second second	ACCRECATION AND ADMINISTRATION OF THE
Depth (ft.)	Strata Description	Class.	Str'm	Elev.	InS Depth	itu Testing Geo. Pen.	M (%)	Remarks
	5" topsoil gray, brown silty sand with gravel – FILL	SM	F	887.5			15.2	infiltration testing @ 0.5'; El 888.0
	brown, gray silty SAND	SM	В		_			,
2					2 —		16.2	
				884.5				
4	Bottom of Test Pit at 4.0 feet			664.3	4			
6					6 —			
8					8 —			
10 —					10 —			
12					12 —			
14					14			
16					16			
18					18			
20 —					12 —			
20					14			



roject: Milford Warehouse Development

Milford Township, PA

Test Loc. No.: **TP-15**

Contract No.: 23004

Date Excavated: 08/03/23

ME, Inc. Rep.: DH

Equip. Used: Excavator

Surface Elev: 911.7

Groundwater Observations

Encountered: 08/03/23 Depth: none Completion: 08/03/23 Depth: dry

Comments: Backfilled upon completion.

TEST PIT LOG TP-15



Depth (ft.)	Strata Description	Class.	Str'm	Elev.	InSit Depth	tu Testing Geoprobe Penetr'n	M (%)	Remarks
	6" topsoil					Tened ii		
1					1			
2 —	brown poorly graded GRAVEL with	GP	В		2 —			
3	sand, cobbles				3 ——			
4					4		2.6	
5 —					5 ——			
6					6 —			
7 —					7 —			
8					8			
9 —					9 —			
10					10 —		4.0	
11					11			
12 —					12			
13					13 —			
14					14			
15 —					15 —			
16-	Bottom of Test Pit at 16.0 feet			895.7	16			
17					17 —			
- '					1,			



Project: Milford Warehouse Development

Milford Township, PA

Test Loc. No.: TP-16

Contract No.: 23004

Date Excavated: 08/03/23

ME, Inc. Rep.: DH

Equip. Used: Excavator

Surface Elev: 906.8

Groundwater Observations

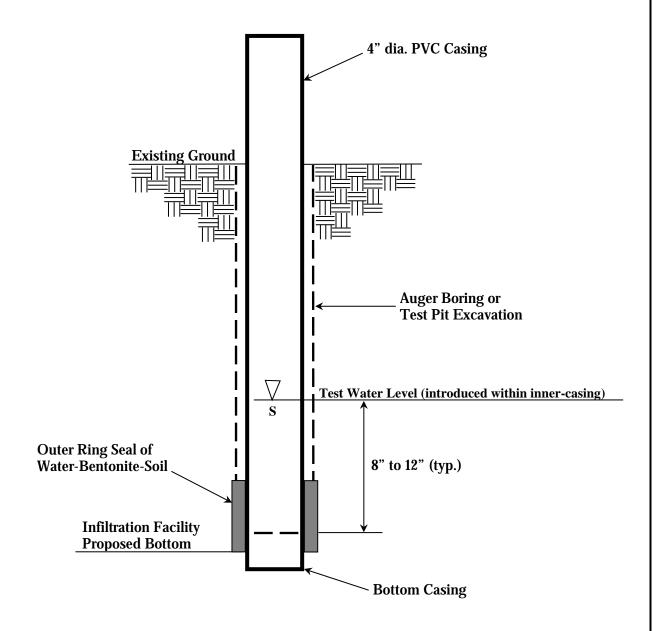
Encountered: 08/04/23 Depth: none Completion: 08/04/23 Depth: dry

TEST PIT LOG TP-16



Depth (ft.)	Strata Description	Class.	Str'm	Elev.	InSi Depth	tu Testing Geoprobe Penetr'n	M (%)	Remarks
	topsoil					reneu n		
1					1			
					2			
2 —	brown poorly graded GRAVEL with	GP	В		2 —			
3 —	sand, cobbles				3 —			
4					4		3.5	
5 —					5 —			
6					6 —			
7 —					7 —			
8					8 —			
9 —					9 —			
10					10 —			
11					11			
12					12			
13	brown poorly graded SAND with gravel, cobbles	SP			13 —			
14					14		3.0	
15					15 —			
16					16 —			
17 —	Bottom of Test Pit at 17.0 feet			889.8	17 —			

IN-SITU INFILTRATION TESTING





120 Commerce Road • Pittston Township, PA 18640-9552 570/655-2200 (phone) • midlaneng@aol.com

Drawing Title:

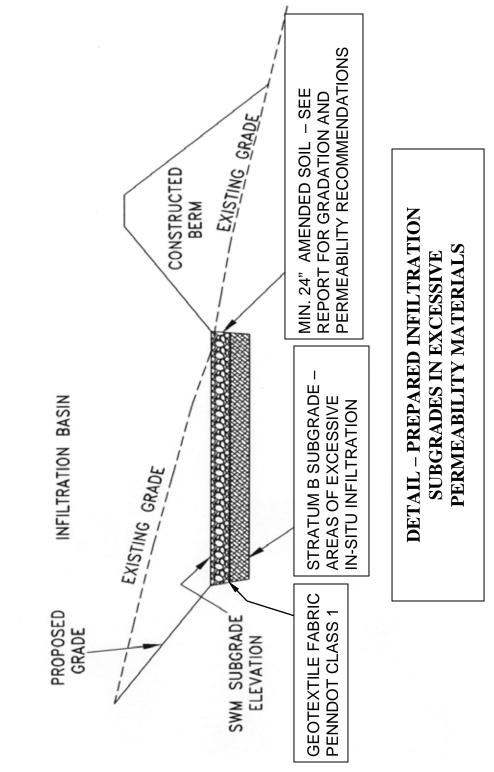
In-situ Infiltration Testing Setup

Milford Warehouse Development

Milford Township, PA

Drawn By:	Checked By:	Scale:
LA	TB	NTS
Date:	Project No.:	Sheet No.
08/18/23	23004-SWM	

Figure No. 6-1





120 Commerce Road • Pittston Township, PA 18640-9552 570/655-2200 (phone) • midlaneng@aol.com

Drawing Title:

Detail - Prepared Infiltration Subgrades

Milford Warehouse Development

Milford Township, PA

Drawn By:	Checked By:	Scale:
LA	TB	NTS
Date:	Project No.:	Sheet No.
08/18/23	23004-SWM	